## **Data Reduction**

available data to assess the nature and the degree of the uncertainty. tion, and reduction of observed data which facilitate its interpretation easily assimilated. There are several methods of organization, presenta-An unorganized list of numbers representing the outcomes of tests is not A necessary first step in any engineering situation is an investigation of and evaluation.

which we shall come to know and which some readers may have encounsample of some mathematical probabilistic model. These are terms described in this chapter is in no way dependent on the assumptions that tered previously. The methods here are simply convenient ways to there is randomness involved or that the data constitute a random reduce raw data to manageable forms. It should be pointed out explicitly that the treatment of data

tudes of variation to be expected in civil-engineering problems. problems, it is important that the reader appreciate that the data are "real" and that the variability or scatter is representative of the magni-In studying the following examples and in doing the suggested

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## 1.1 GRAPHICAL DISPLAYS

observed in a New York warehouse. To anticipate the typical, the pounds per square foot (psf). Let us divide this range into 20-psf settlements of the column footings. extreme, and the long-term behavior of structural members and footings presented in Table 1.1.1. These numbers represent the live loads Histograms A useful first step in the representation of observed data is in each interval. intervals, 0 to 19.9, 20.0 to 39.9, etc., and tally the number of occurrences distribution. Load variability will, for example, influence relative in such structures, the engineer must understand the nature of load to reduce it to a type of bar chart. Consider, for example, the data The values vary from 0 to 229.5

Plotting the frequency of occurrences in each interval as a bar

Table 1.1.1 Floor-load data\*

Bay	Base- ment	1st	2 <i>d</i>	3.4	4th	5th	The state of the s	6th	6th 7th	
A	0	7.8	36.2	60.6	64.0		64.2	79	79.2	79.2 88.4
B	72.2	72.6	74.4	21.8	17.1	1.6	48.5	16	16.8 105	16.8 105.9
D C	225.7 55.1	42.5 55.9	59.8 87.7	41.7 59 2	39.9		55 55 50 51	55.5 67.2 58.8 67.7	67	67.2 122 67.7 90
E	36.6	26.0	90.5	23.0	43.5		52.1	102	102.1 71	102.1 71.7
F	129.4	66.4	138.7	127.9	90.9		46.9	197	197.5 151	197.5 151.1
G	134.6	73.4	80.9	53.3	80.1		62.9	150	150.8 102	150.8 102.2
H	121.0	106.2	94.4	139.6	152.5		70.2	111	111.8 174	111.8 174.1
Ι	178.8	30.2	44.1	157.0	105.3		87.0	50	50.1 198	50.1 198.0
J	78.6	37.0	70.7	83.0	179.7		180.2	60	60.6 212	60.6 212.4
K	94.5	24.1	87.3	80.6	74.8		72.4	131	131 1 116	131.1 116.1
L	40.2	23.4	8.4	42.6	43.4		27.4	63	63.8 18	63.8 18.4
M	92.2	49.8	50.9	116.4	122.9		132.3	105	105.2 160	105.2 160.3
N	99.5		55.9	136.8	110.4		123.5	92	92.4 160	92.4 160.9
0	88.5		62.3	71.3	133.2		92.1	111	111.7 67	111.7 67.9
Þ	93.2		80.8	143.5	122.3		184.2	150	150.0 57	150.0 57.6
Ø	96.1		63.0	228.3	139.3		59.1	112	112.1 50	112.1 50.9
R	213.7		90.3	198.4	97.5		155.1	163	163.4 155	163.4 155.3
S	137.6		156.5	154.1	134.3		81.6	194	194.4 155	194.4 155.1
T	79.8		85.6	141.6	100.7		106.0	131	131.1 157	131.1 157.4
4 0	78.5	118.2	126.4	. 33 . 33 . 30	124.6		78.9	146	146.0	146.0 100.3
_	24.8		135.6	56.3	66.9		72.2	105	105.4 98	105.4 98.9

\* Observed live loads (in pounds per square foot); bay size: 400 ± 6 ft\*.
Source: J. W. Dunham, G. N. Brekke, and G. N. Thompson [1952], "Live Loads on Floors in Buildings" Building Materials and Structures Report 133, National Bureau of Standards, p. 22.

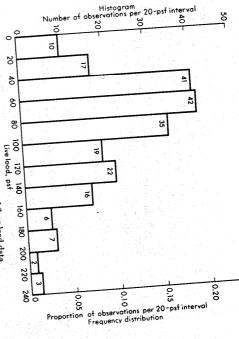


Fig. 1.1.1 Histogram and frequency distribution of floor-load data.

the area, of each bar are proportional to the number of occurrences in yields a histogram, as shown in Fig. 1.1.1. The height, and more usefully, quently occurring values, and the degree to which it is scattered about the gator an immediate impression of the range of the data, its most frethat interval. The plot, unlike the array of numbers, gives the investicentral or typical values. We shall learn in Chap. 2 how the engineer can predict analytically from this shape the corresponding curve for the total load on a column supporting, say, 20 such bays.

number of data entries, an alternate form, called the frequency distribution, results. In Fig. 1.1.1, the numbers on the right-hand scale were obtained by dividing the left-hand scale values by 220, the total number of observathe interval length (20 psf), a frequency density distribution would result, to lie between 120 and 139.9 psf was 0.10. with ordinate units of "frequency per pst." The area under this histogram would be unity. This form is preferred when different sets of data, perhaps with different interval lengths, are to be compared with one If the scale of the ordinate of the histogram is divided by the total One can say, for example, that the proportion of loads observed If this scale were divided by

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lines to form a nondecreasing (or monotonic) function from zero to unity division point. These points are then plotted and connected by straight culating the successive partial sums of frequencies up to each interval sentation of data, is obtained from the frequency distribution by cal-The cumulative frequency distribution, another useful graphical repre-

0.1230, and 0.1230 + 0.1860 = 0.3090.† From this plot, one can read psf was 0,847. After a proper balancing of initial costs, consequences of load data, the values of the function at 20, 40, and 60 psf were found by should be checked for deflections under a load of 220 psf. forming the partial sums 0 + 0.0455 = 0.0455, 0.0455 + 0.0775 =deflections in excess of 1 in. in 99 percent of all bays. Thus the design that a beam supporting one of these bays must be stiff enough to avoid poor performance, and these frequencies, the designer might conclude that the proportion of the loads observed to be equal to or less than 139.9 In Fig. 1.1.2, the cumulative frequency distribution of the floor-

i/n versus  $x^{(i)}$ , where  $x^{(i)}$  is the *i*th in ordered list of data (see Fig. 1.2.1). arbitrariness of the intervals by plotting one point per observation, that is, by plotting † When constructing the cumulative frequency distribution, one can avoid the Some care should be taken in choosing the width of each interval

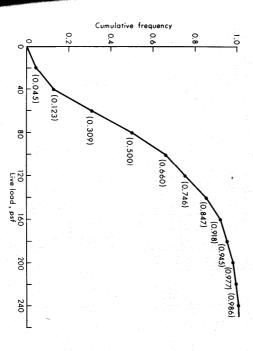


Fig. 1.1.2 Cumulative frequency distribution of floor-load data.