

Θαλάσσια Υδραυλική και Λιμενικά Έργα

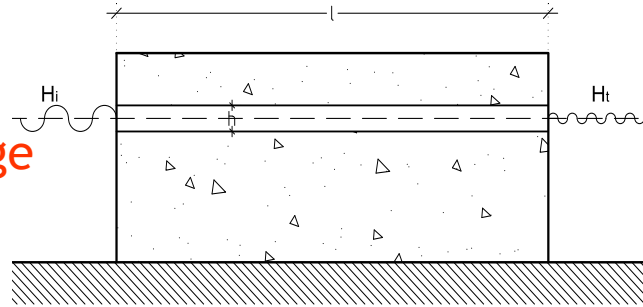
Ειδικά θέματα Λιμενικών Έργων: Αγωγοί Ανανέωσης

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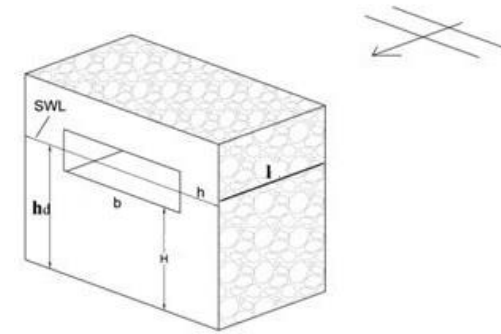
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Flushing Culverts

- ▶ Water quality within the basin preserved **water periodic exchange** between harbour and open sea
- ▶ placement of **flushing culverts** across the breakwaters
- ▶ The axis of the culverts, in regions where the ranges of tides are low (e.g. the Mediterranean), is constructed at the sea water level
- ▶ **Flushing culverts** allow water recirculation:
 - [+] ensure water quality
 - [-] increase the wave disturbance into the basin (problem with: *navigation, mooring of vessels*)



Vertical-front breakwater bearing a flushing culvert



Marina Kalamata



Αγωγοί Ανανέωσης στον Ελλαδικό χώρο

- 600 λιμάνια στην Ελλάδα, ανάγκη για αγωγούς ανανέωσης
- Επικρατεί η ανεμογενής κυκλοφορία των υδάτων → διαμήκης άξονας στη στάθμη ηρεμίας ύδατος



Κυματοθραύστης με αγωγό ανανέωσης στην Καλαμάτα.



Κυματοθραύστης με αγωγό ανανέωσης στο Μάτι.



Αγωγοί ανανέωσης στη Μυτιλήνη.



Flushing Culverts

- ▶ Designing a flushing culvert → good understanding of the hydrodynamic :
 - *partial reflection*
 - *diffraction*
 - *production of evanescent modes*
 - *wave breaking*
 - *generation of higher order harmonics*
- ▶ Main non-dimensional parameters concerning the water quality within the harbour basin → the wave transmission coefficient, K_t

$$K_t = H_t/H_i$$

H_t :transmitted wave height through the flushing culvert

H_i :incident wave height

- ▶ The K_t is correlated with:
 - *the wave characteristics*
 - *the geometrical characteristics of the culvert*
 - *the water depth*

Numerical Methods

- ▶ Boussinesq models not appropriate as they assume a depth-average velocity profile → does not allow the detailed description of the evanescent modes that arise due to the abrupt changes in water depths.
- ▶ Boundary element methods (BEM) → good profile description and very effective in the isolation of each harmonic
- ▶ Reynolds Averaged Navier-Stokes RANS → and change in mean water level
 - ▶ Volume- Averaged - (VA)RANS e.g. Garcia and Liu, 1998
 - ▶ Large Eddy Simulation (LES) e.g. Losada
- ▶ 2D linear coupled-mode system Athanassoulis & Belibassakis (1999)
- ▶ extended to second-order Belibassakis & Athanassoulis (2002)
 - ▶ Limitations concerning nonlinearity of the wave field & as the culvert width decreases
- ▶ 3D linear coupled-mode model, based on eigen-functions expansions of the Laplace equation Belibassakis, Tsoukala & Katsardi (2014)
 - ▶ Much improved compared with 2D concerning diffraction but again limitations concerning nonlinearity

- Computationally expensive
- Understanding of physics rather than investigate the effectiveness of FCs

The development of Artificial Neural Networks- ANNs

Tools that combine the experimental results with some kind of error correction algorithm → accomplish a more accurate prediction of the target value

- ▶ Van den Boogaard et al. (2009) → numerical model, powered by a limited number of experimental measurements - calculate the loads exerted on the toe of breakwaters under critical wave conditions

Closer to the present work:

- ▶ Panizzo and Briganti (2007) → ANN fed by a large number of experimental measurements, estimating wave transmission coefficient over low-crested structures
- ▶ Castro et al. (2011) → investigated the reflection coefficient of submerged breakwaters by using ANN
- ▶ Chondros and Memos (2012) → ANN based on a small experimental database & a Boussinesq-type model - derive a compound tool for prediction of wave transmission coefficient over submerged breakwaters

Semi-empirical Formulas

Tsoukala et al. (2010) - experimental measurements, were carried out in the wave flume at the Laboratory of Harbor Works at the National Technical University of Athens

$$K_t = \frac{H_t}{H_i} = \left(0.135 \frac{b}{H_i} + 0.048 \frac{2h_s}{H_i} 0.030 \frac{h_f}{H_i} 0.026 \frac{d}{H_i} 0.036 \frac{l}{H_i} \right) * \mu^{-0.681}$$

h_f : the water depth at the toe of the flushing culvert

b : the width of the flushing culvert

h_s : the half of the height of the flushing culvert

d : the water depth

l : the length of the flushing culvert

H_i : the incident wave height

T : the wave period

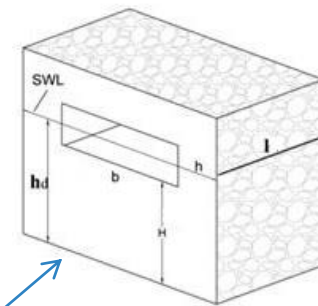
$\tan\alpha$: the breakwater's slope

$$\mu = \sin\alpha / \sqrt{2\pi H_i / g T^2}$$

Experimental Data

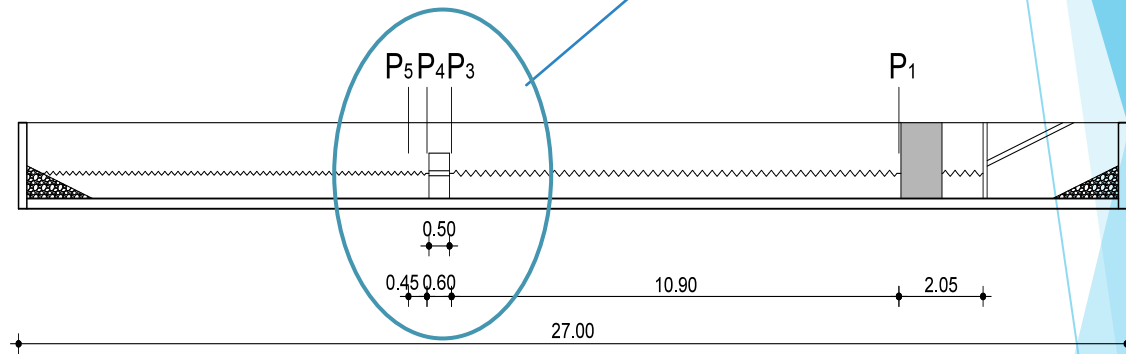
2D and 3D experimental facilities

Giannaki (2005), Tsoukala et al. (2010) and Chondros et al (2014)



The 2D wave flume:

- 27.00 m long
- 0.60 m wide
- 1.53 m depth



The scaled *breakwater* with a vertical forehead:

- 1.00 m height
- 0.50 m length
- 0.60 m width

- 0.60 m water depth

Side-view of the wave flume and the experimental setup showing wave probe locations

time series of water surface elevation were measured

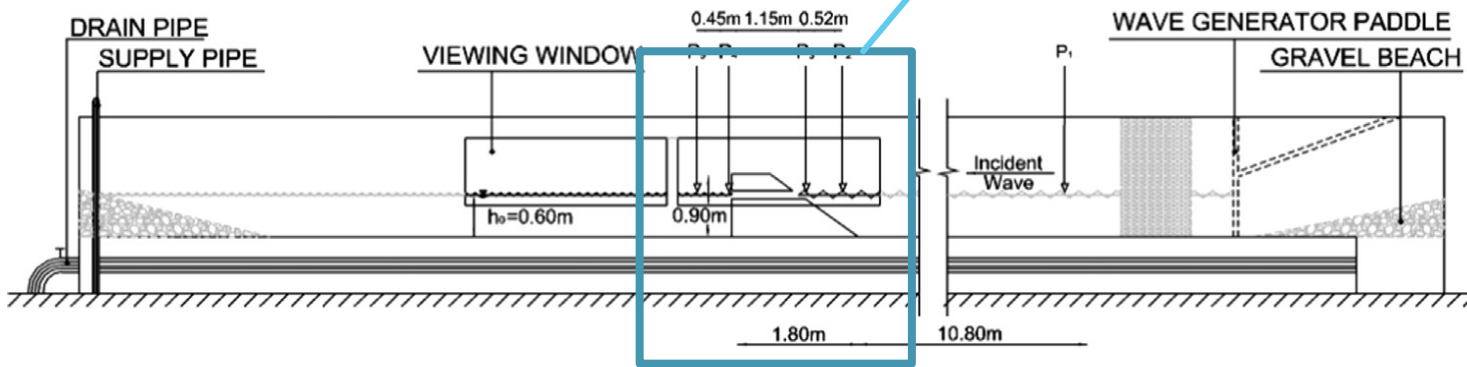
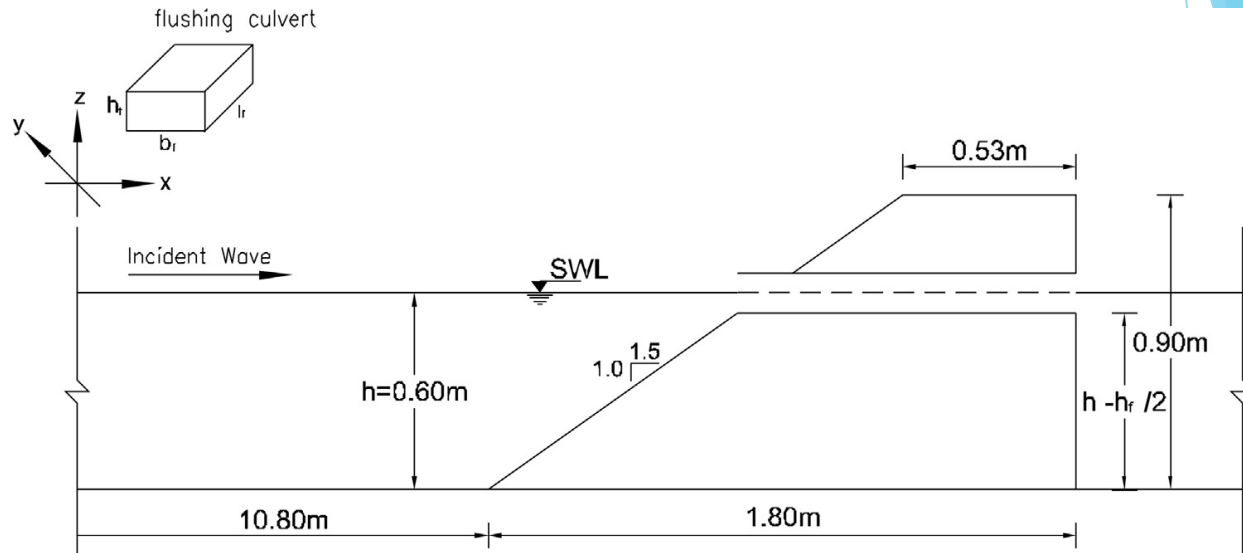
Experimental Data - 2D wave flume

The flushing culvert - with the center of their axes at the SWL:

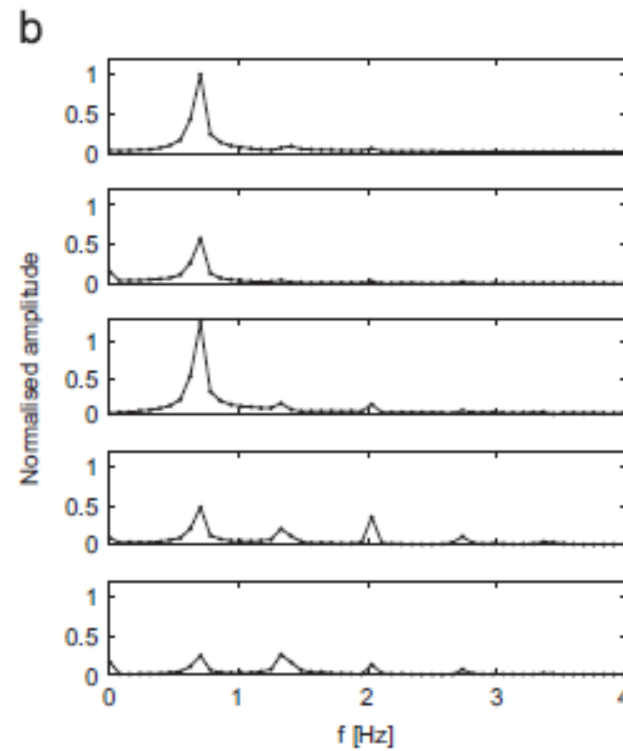
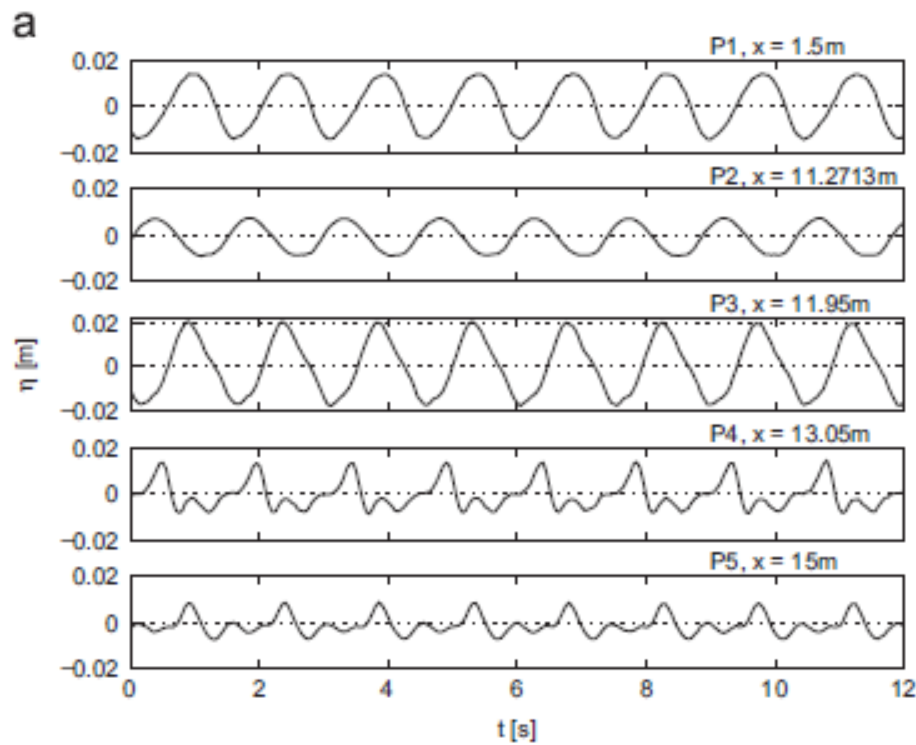
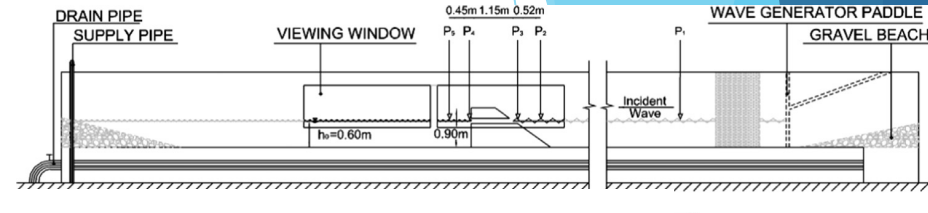
Case	h	l	b
	cm	cm	cm
A	12	50	18
B	12	50	24

The measurements were performed for:

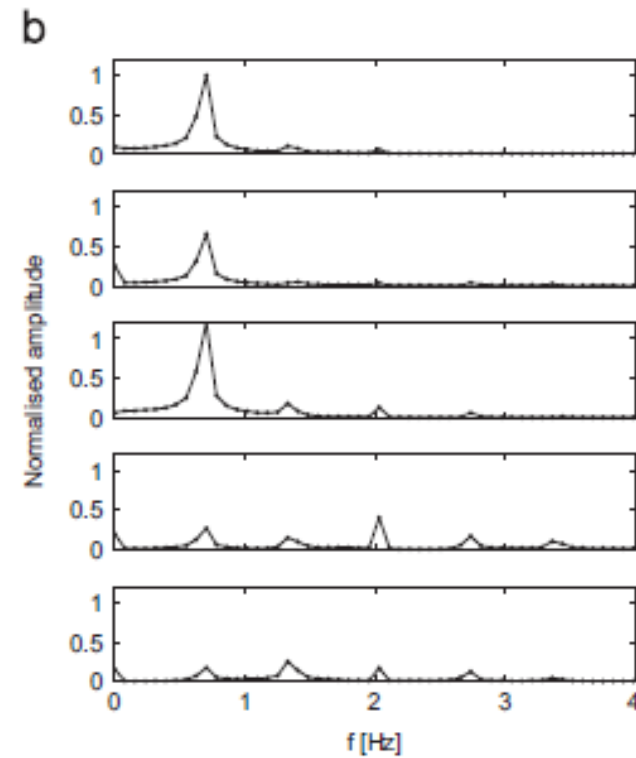
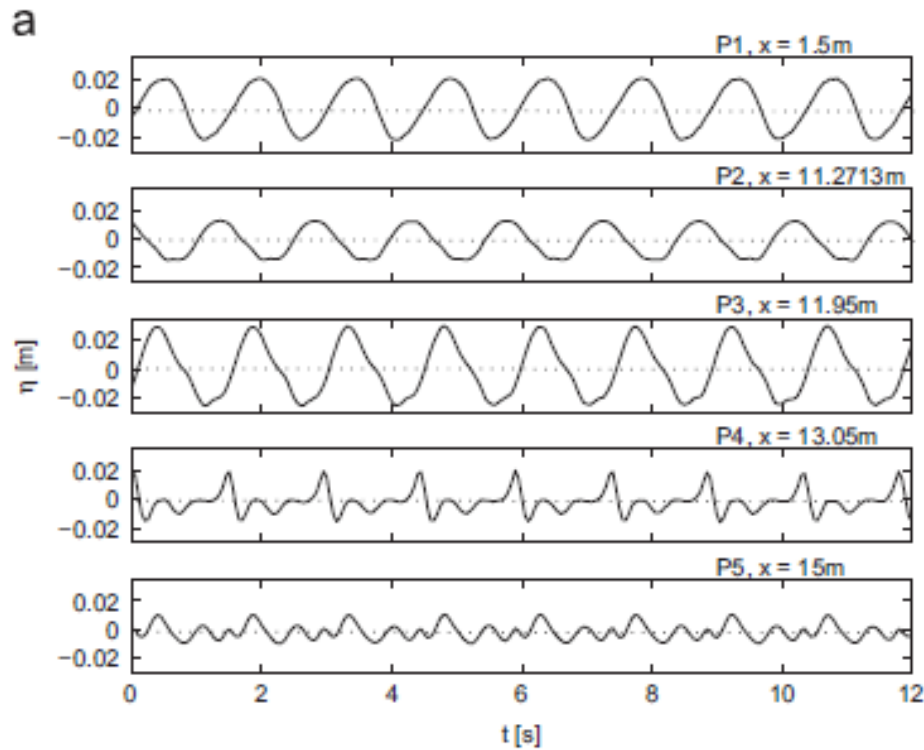
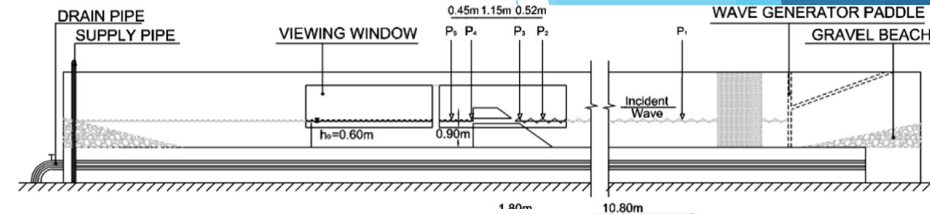
- 15 different incident regular waves
- 3 different periods [T= 1.48s 1.10s and 0.90 s]
- variation of the mean wave height $\rightarrow 2 \leq H \leq 13$ cm



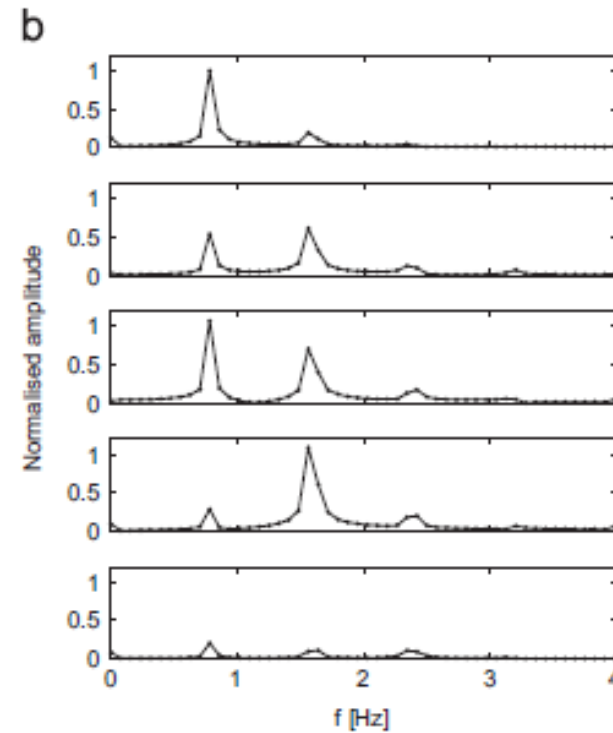
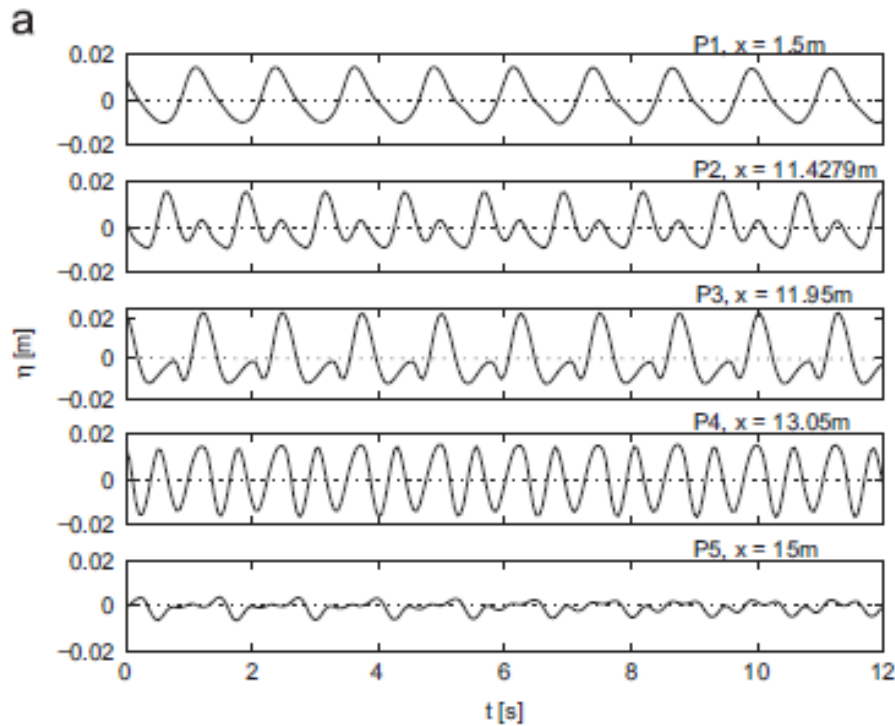
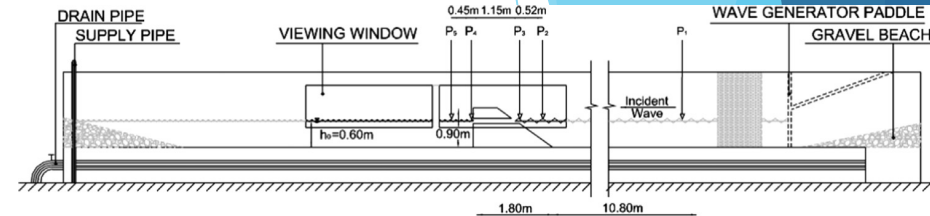
Case 7: $T=1.47\text{s}$, $H=0.028\text{ m}$, $hf=0.18\text{m}$, $bf=0.24\text{ m}$



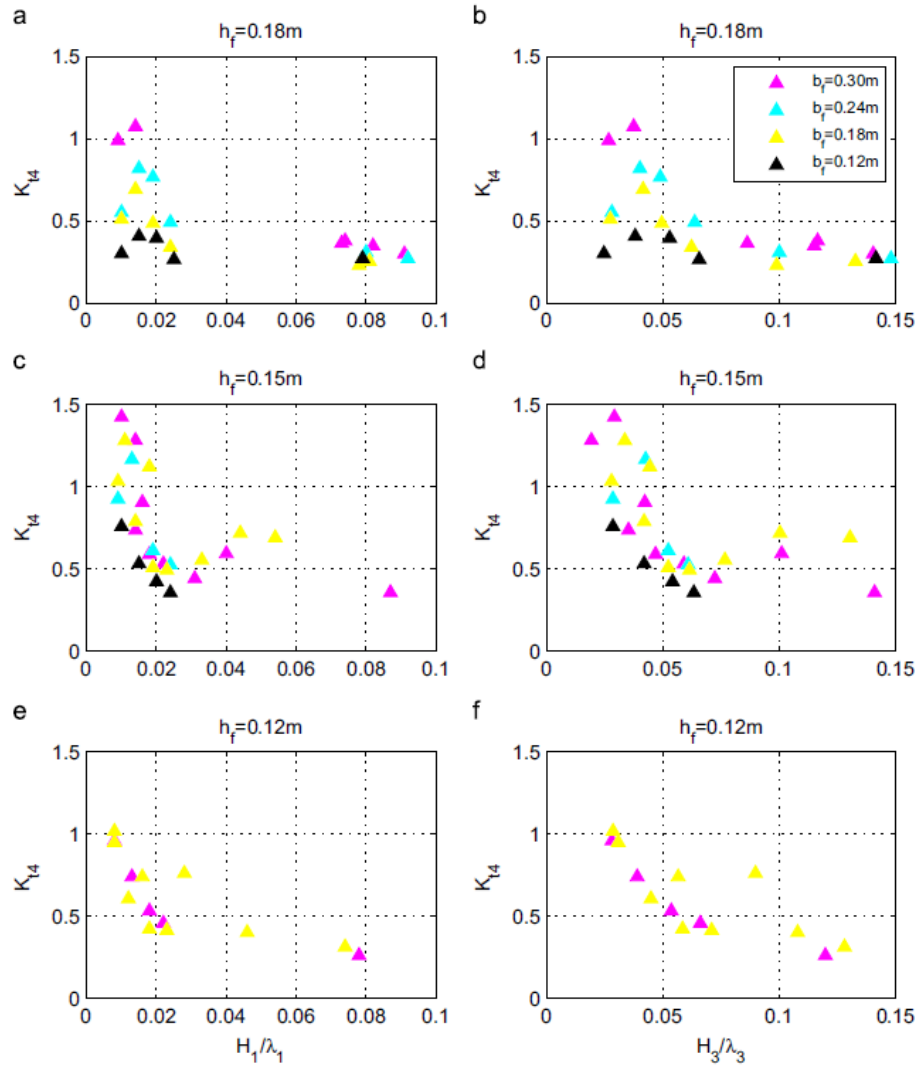
Case 7: $T=1.47\text{s}$, $H=0.043\text{m}$, $hf=0.18\text{m}$, $bf=0.24\text{m}$



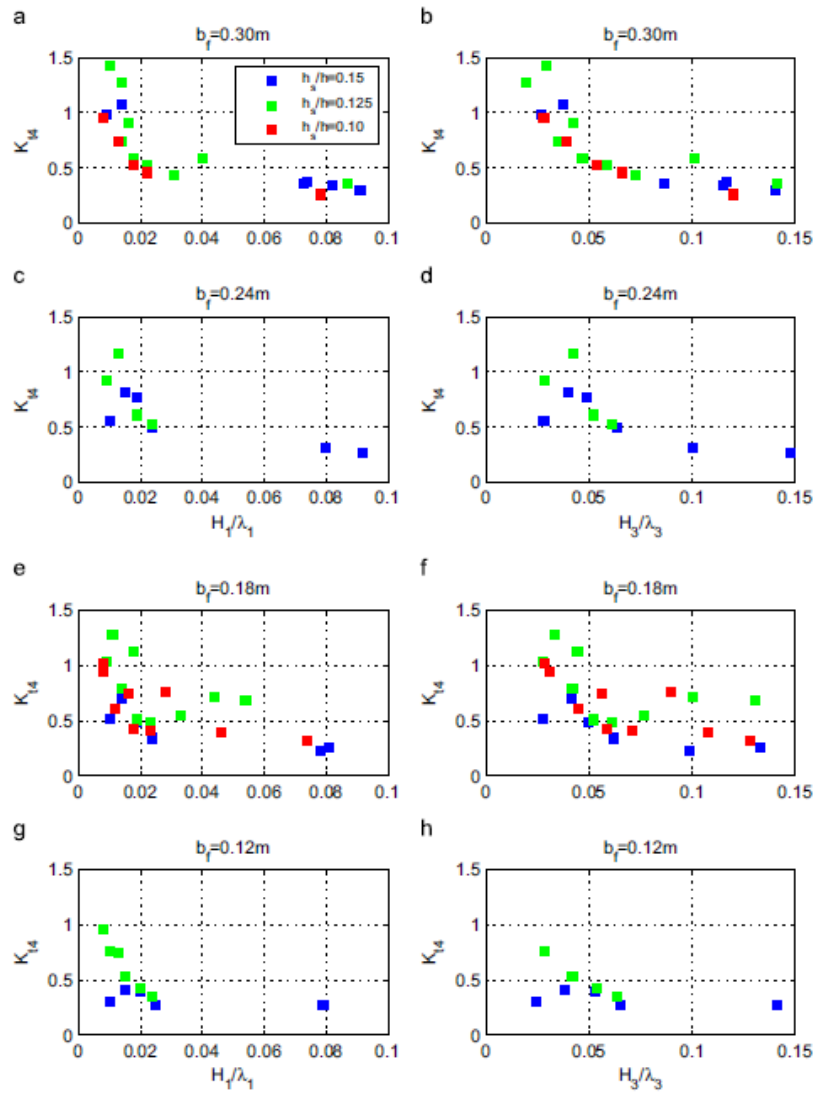
Case 7: $T=1.26\text{s}$, $H=0.025\text{m}$, $hf=0.15\text{m}$, $bf=0.18\text{m}$



Transmission Coefficient

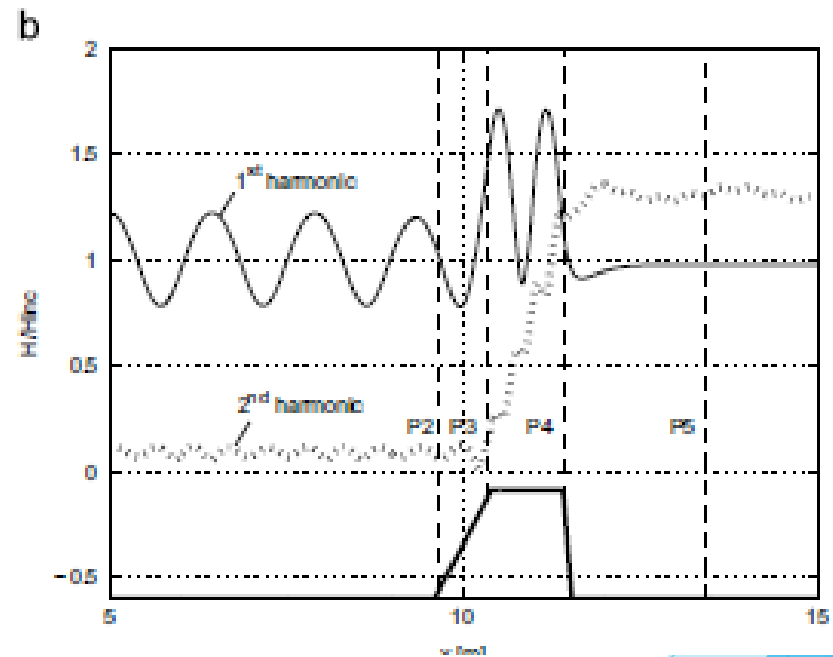
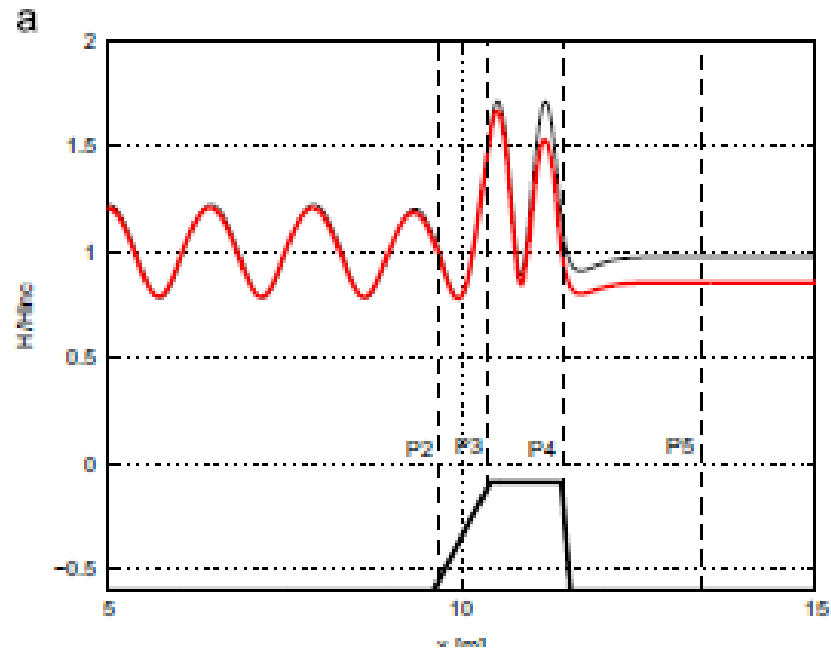


Transmission Coefficient



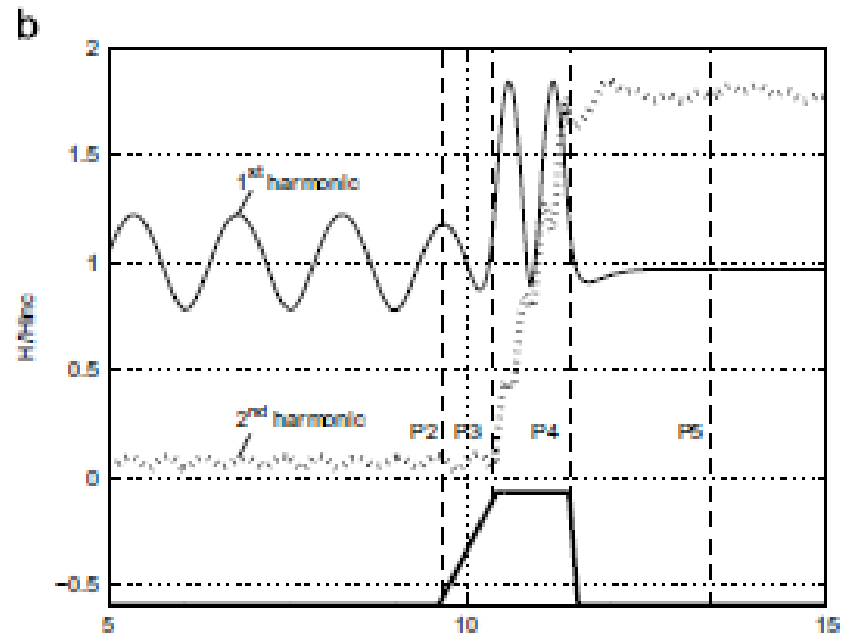
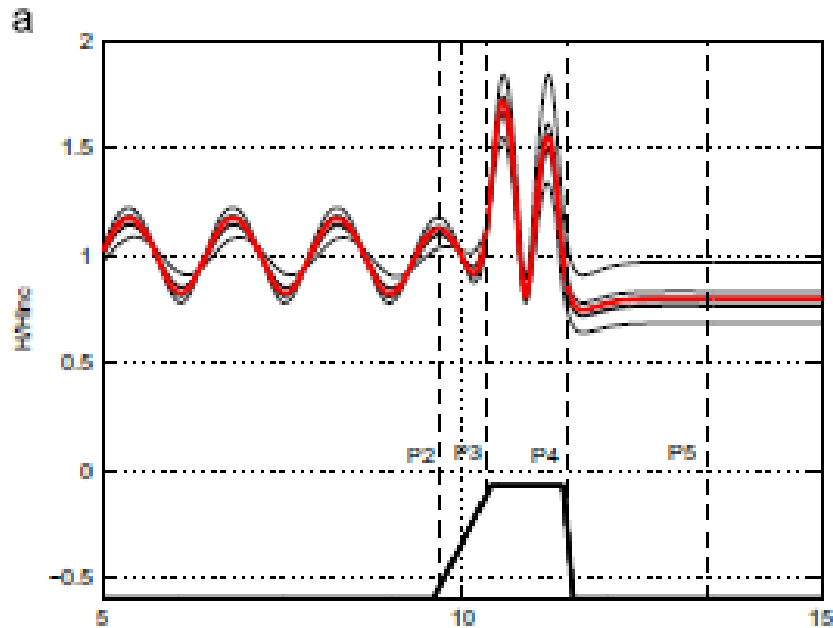
CMS calculations

$T=1.47\text{s}$, $H=0.027\text{m}$, $hf=0.18\text{m}$

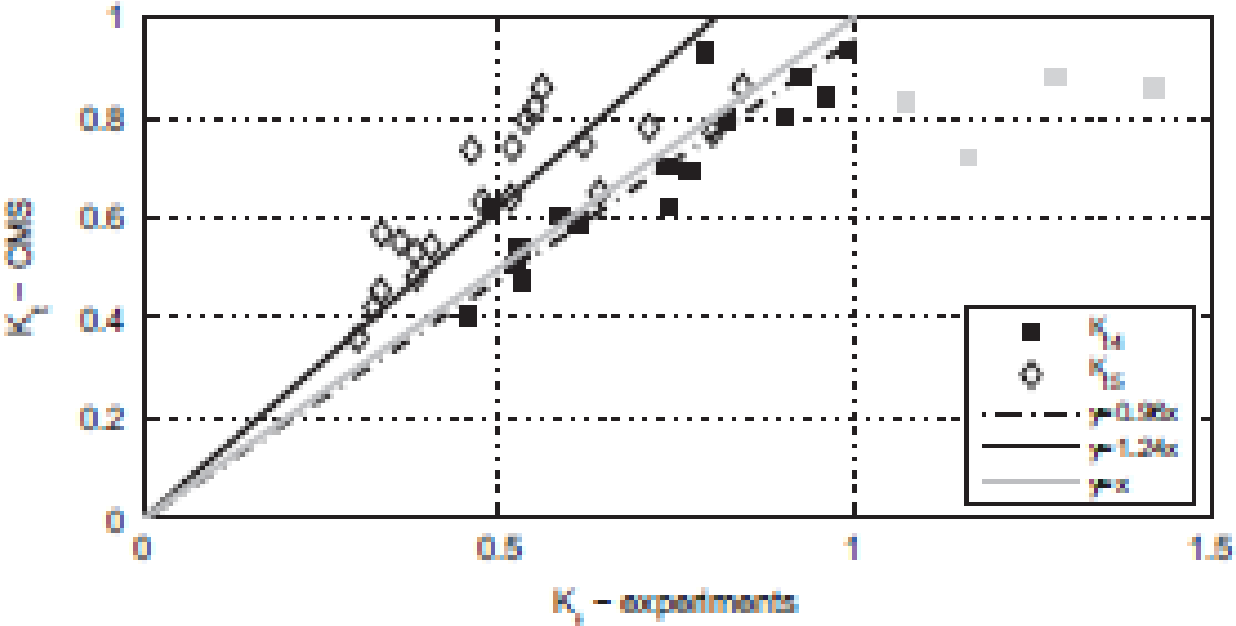


CMS calculations

$T=1.47\text{s}$, $H=0.027\text{m}$, $hf=0.15\text{m}$



CMS-Experiments



Thank you for your attention..!